

ACACIA LODGE NO. 16 A.F. & A.M.

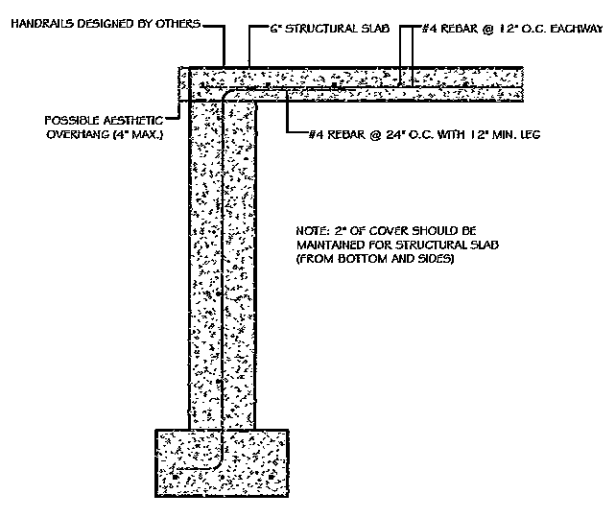
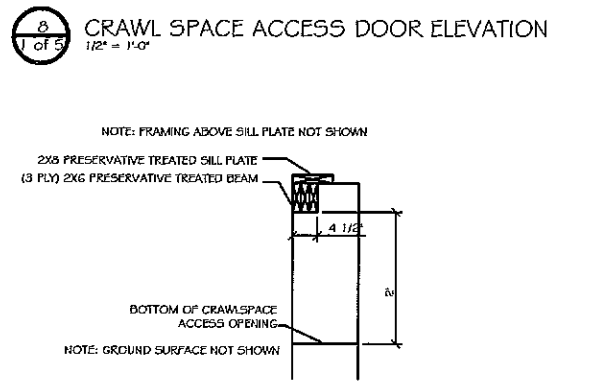
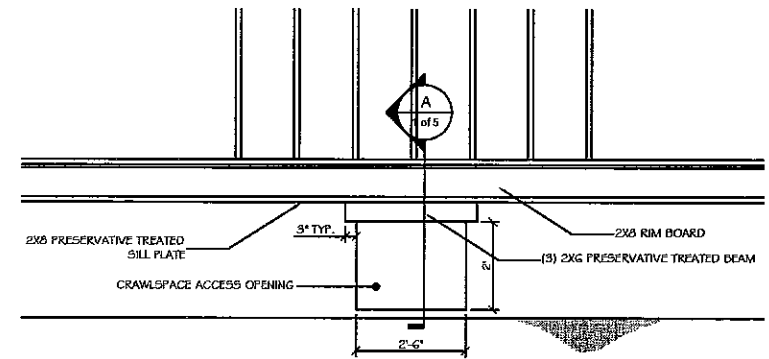
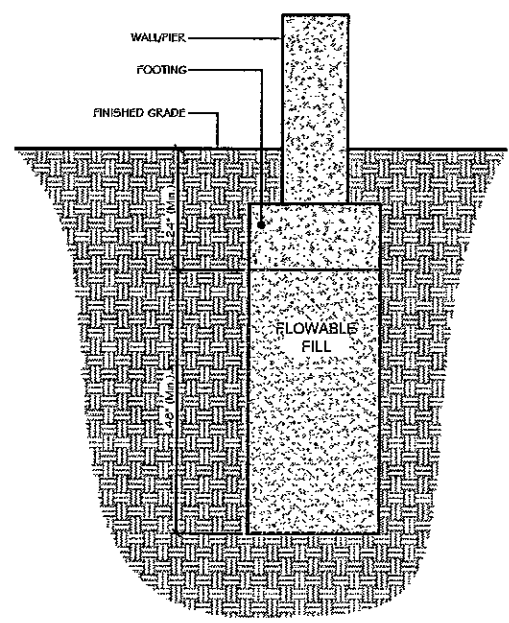
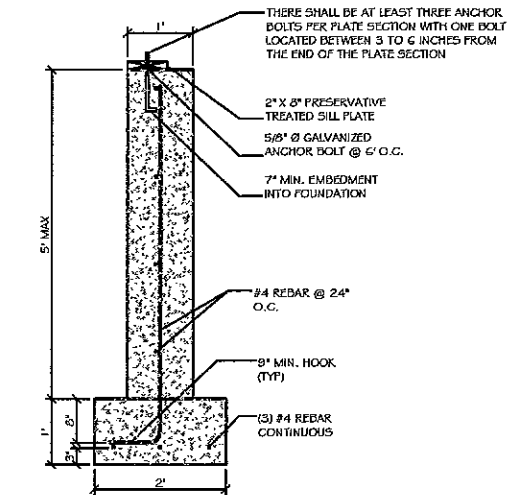
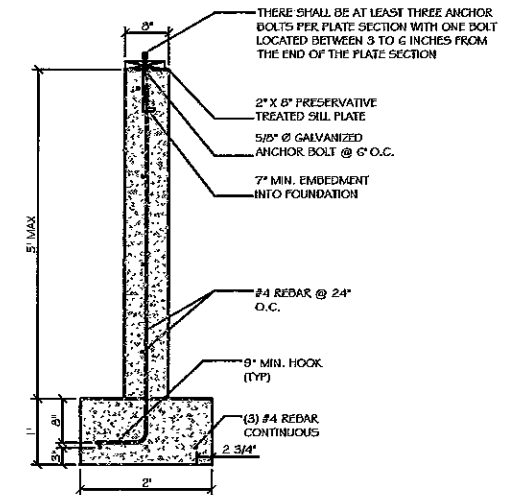
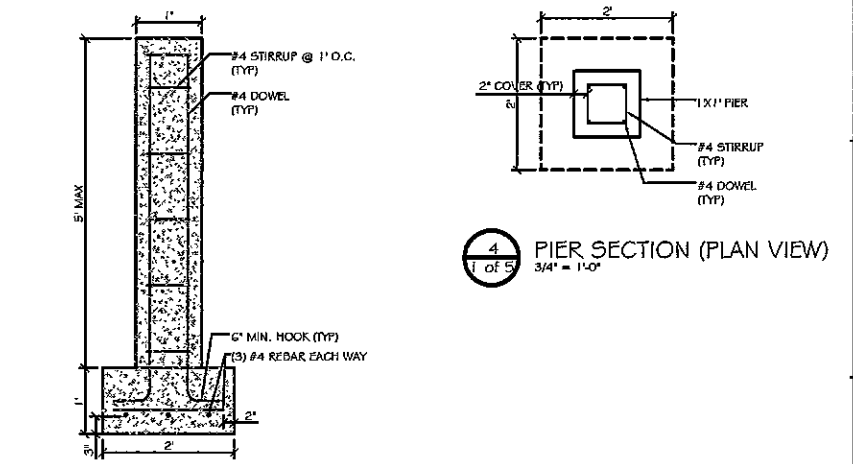
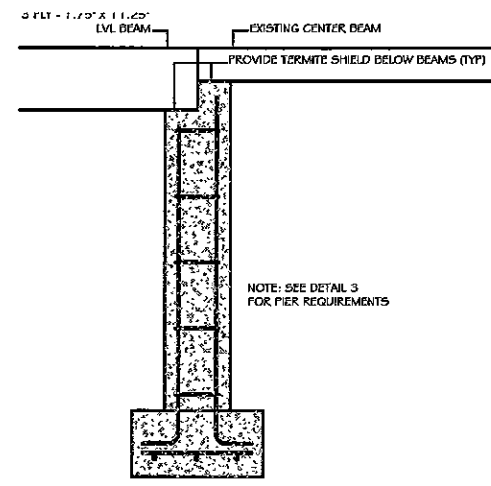
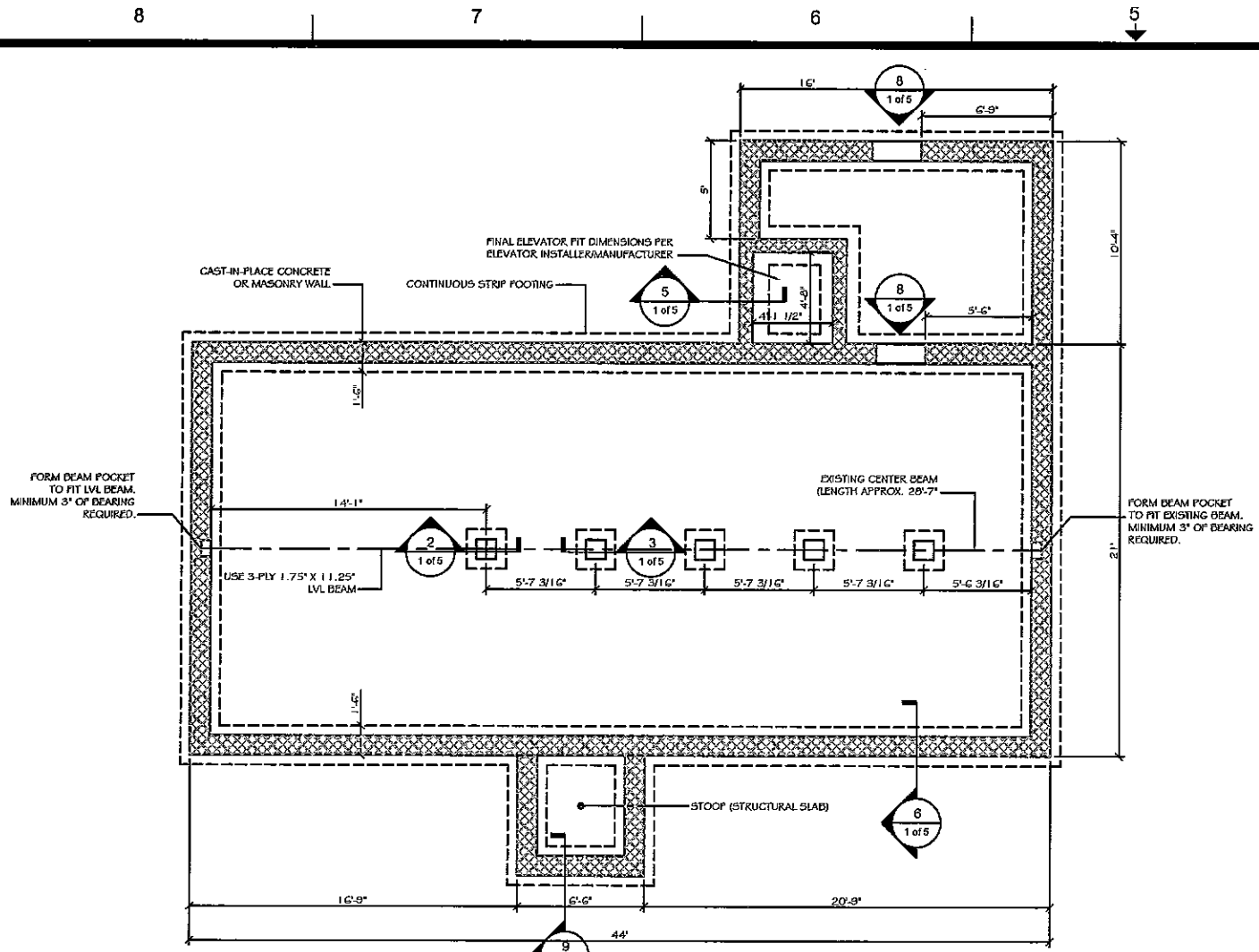
CLIFTON, VIRGINIA



DOMINION ENGINEERING ASSOCIATES, INC.
5110 SOUTHPOINT PARKWAY
FREDERICKSBURG, VA 22407

SHEET INDEX:

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- FOUNDATION NOTES:**
- TOP OF FOUNDATION WALL SHALL NOT EXCEED ELEVATION 187.00'
 - NEED TO CAST SMART VENTS (OR EQUIVALENT) WITH WALL POUR, OR BOX OUT FOR FUTURE INSTALLATION.
 - IF SIDEWALLS OF FOUNDATION EXCAVATION CAVE, VDOT NO. 57 STONE MAY BE USED IN LIEU OF FLOWABLE FILL, OR AS OTHERWISE RECOMMENDED BY THE GEOTECHNICAL ENGINEER.
 - CONTRACTOR, AT HIS OPTION, MAY PLACE VERTICAL HOOKS IN FOOTING AND SPLICE VERTICAL WALL BARS WITH MINIMUM SPLICE LAP OF 24".

DOMINION Engineering Associates, Inc.

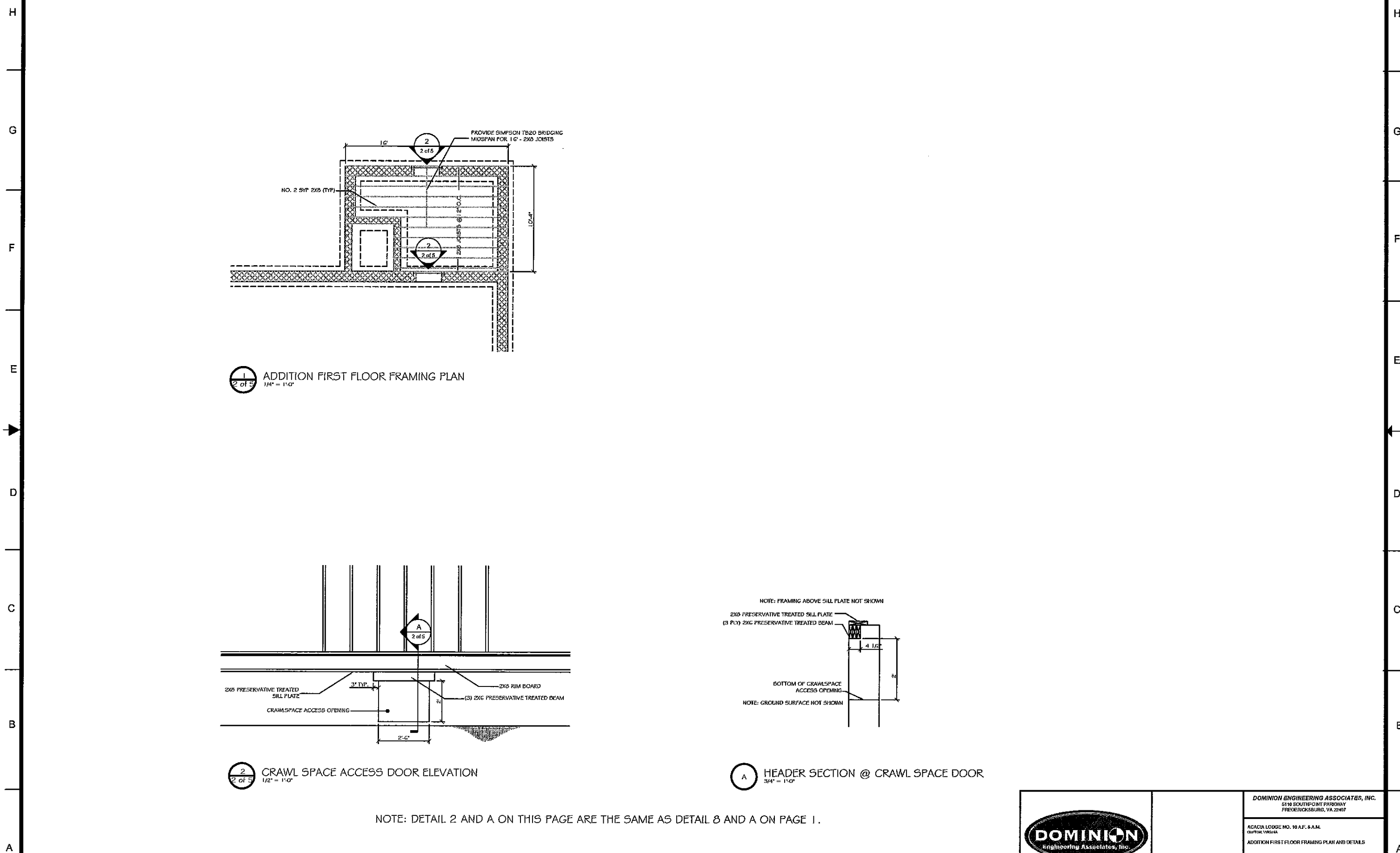
DOMINION ENGINEERING ASSOCIATES, INC.
5110 SOUTHPOINT PARKWAY
FREDERICKSBURG, VA 22407

ACACIA LODGE NO. 16 A.F. & A.M.
CLYDEBORO, VIRGINIA

FOUNDATION PLAN AND DETAILS

DATE: 4 AUG 2008	SHEET: 1 OF 5	SCALE: VARIES
DESIGNED BY: JD	DRAWN BY: JB	CHECKED BY: RH

8 7 6 5 4 3 2 1



1 ADDITION FIRST FLOOR FRAMING PLAN
1/4" = 1'-0"

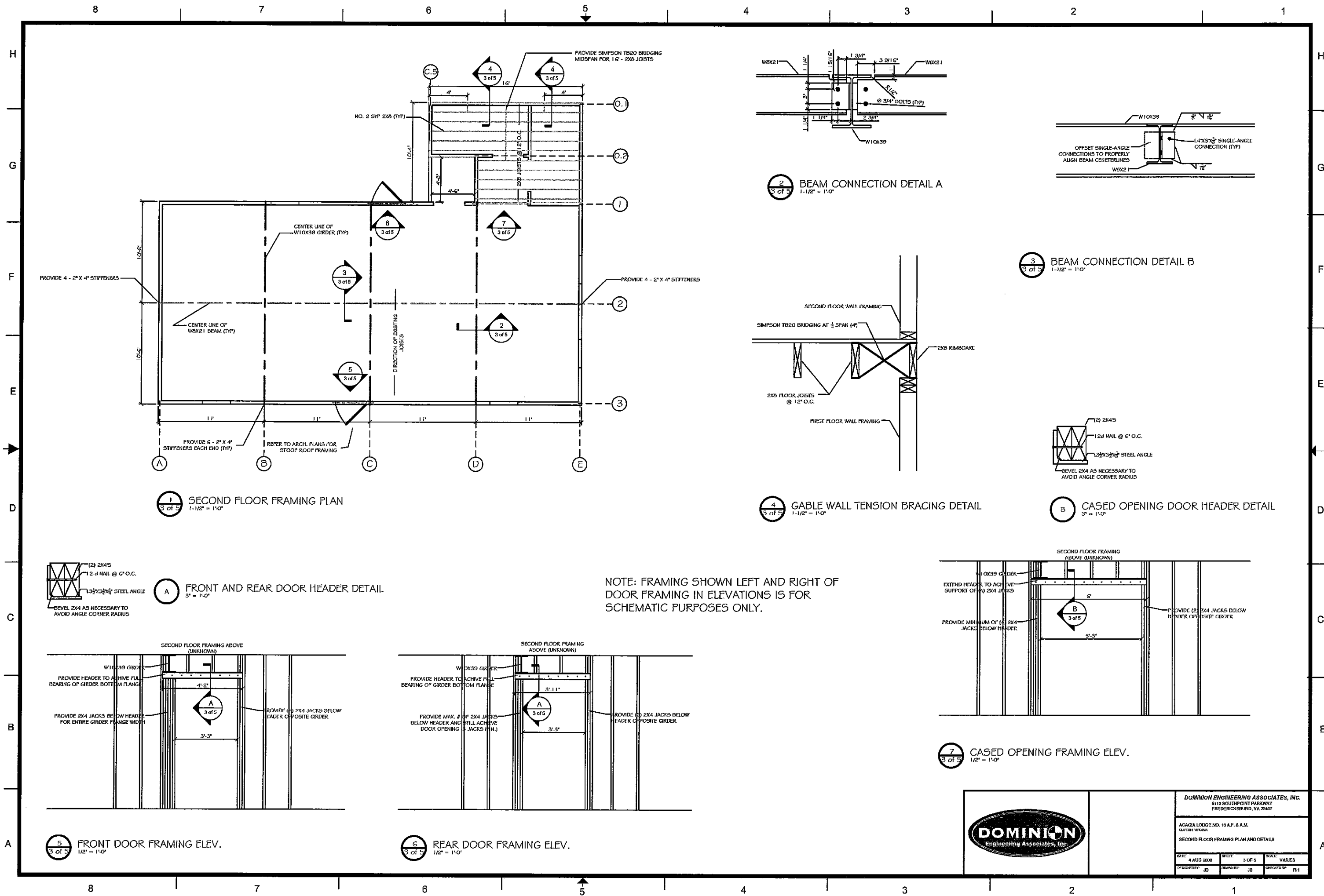
2 CRAWL SPACE ACCESS DOOR ELEVATION
1/2" = 1'-0"

A HEADER SECTION @ CRAWL SPACE DOOR
3/4" = 1'-0"

NOTE: DETAIL 2 AND A ON THIS PAGE ARE THE SAME AS DETAIL 8 AND A ON PAGE 1.

			DOMINION ENGINEERING ASSOCIATES, INC. 5110 SOUTHPOINT PARKWAY FREDERICKSBURG, VA 22407		
			ACACIA LODGE NO. 18 A.F. & A.M. CLYTON, VIRGINIA ADDITION FIRST FLOOR FRAMING PLAN AND DETAILS		
DATE	DRAWN BY	CHECKED BY	SHEET	SCALE	VARIABLES
4 AUG 2008	JD	JB	2 OF 5	VARIES	

8 7 6 5 4 3 2 1



1 SECOND FLOOR FRAMING PLAN
1-1/2" = 1'-0"

2 BEAM CONNECTION DETAIL A
1-1/2" = 1'-0"

3 BEAM CONNECTION DETAIL B
1-1/2" = 1'-0"

4 GABLE WALL TENSION BRACING DETAIL
1-1/2" = 1'-0"

6 CASED OPENING DOOR HEADER DETAIL
3" = 1'-0"

5 FRONT AND REAR DOOR HEADER DETAIL
3" = 1'-0"

NOTE: FRAMING SHOWN LEFT AND RIGHT OF DOOR FRAMING IN ELEVATIONS IS FOR SCHEMATIC PURPOSES ONLY.

7 CASED OPENING FRAMING ELEV.
1/2" = 1'-0"

8 FRONT DOOR FRAMING ELEV.
1/2" = 1'-0"

9 REAR DOOR FRAMING ELEV.
1/2" = 1'-0"



DOMINION ENGINEERING ASSOCIATES, INC. 6110 SOUTHPOINT PARKWAY FREDERICKSBURG, VA 22407		
ACACIA LODGE NO. 19 A.P. & A.M. CLYDE, VIRGINIA		
SECOND FLOOR FRAMING PLAN AND DETAILS		
DATE: 4 AUG 2008	SHEET: 3 OF 5	SCALE: VARIES
DESIGNED BY: JD	DRAWN BY: JB	CHECKED BY: RH

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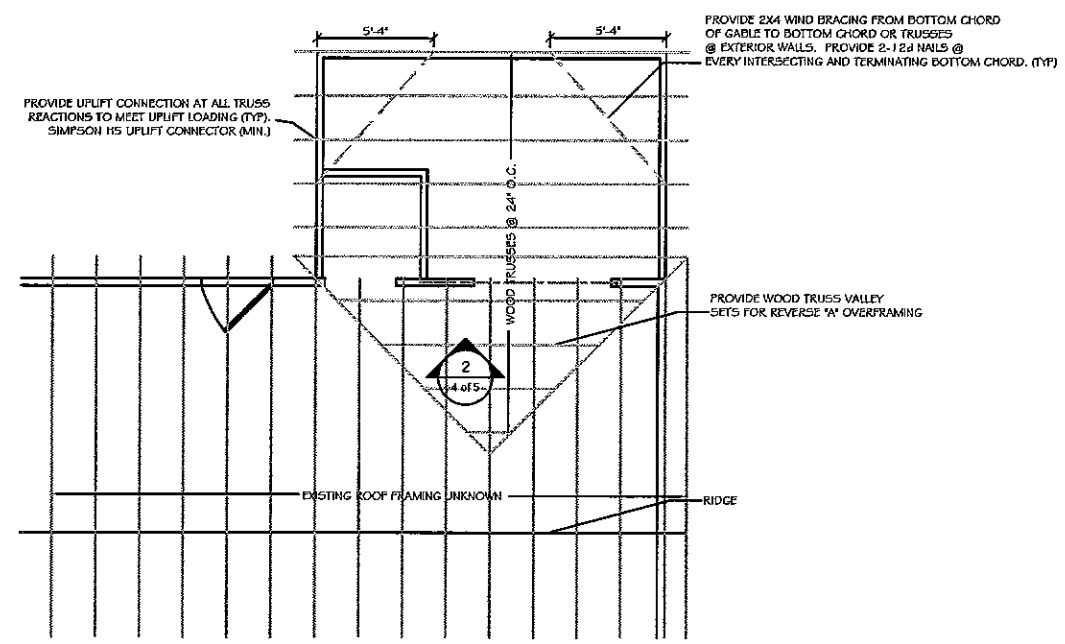
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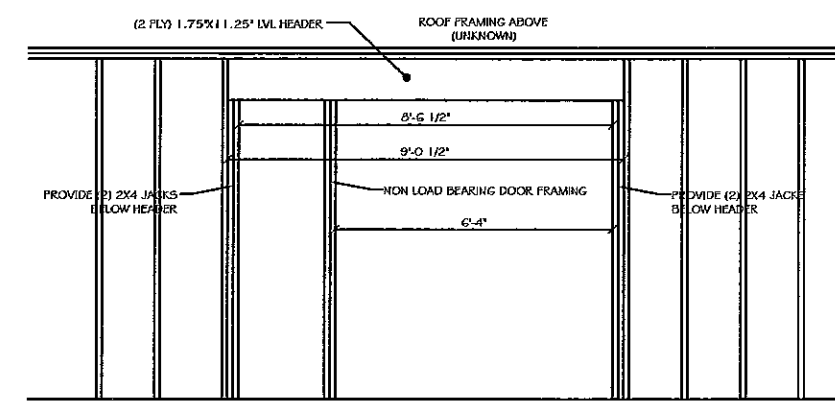
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1
4 of 5
ADDITION ROOF FRAMING PLAN
1/4" = 1'-0"

NOTE: FRAMING SHOWN LEFT AND RIGHT OF DOOR FRAMING IN ELEVATIONS IS FOR SCHEMATIC PURPOSES ONLY.



2
4 of 5
DOUBLE DOOR FRAMING ELEV.
1/2" = 1'-0"



DOMINION ENGINEERING ASSOCIATES, INC. 5110 SOUTHPOINT PARKWAY FREDERICKSBURG, VA 22407			
ACACIA LODGE NO. 10 A.F. & A.M. CLINTON, VIRGINIA			
ADDITION ROOF FRAMING PLAN AND DETAILS			
DATE: 4 AUG 2008	SHEET: 4 OF 5	SCALE: VARES	
DESIGNED BY: JD	DRAWN BY: JB	CHECKED BY: RH	

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PART 1 - SOILS

1.0 GENERAL REQUIREMENTS

1.1 TESTING RESPONSIBILITIES OF THE CONTRACTOR

1.1.1 CONTRACTOR SHALL BE RESPONSIBLE FOR CALLING TESTING AGENCY AT LEAST 24 HOURS PRIOR TO FOUNDATION SUBGRADE INSPECTIONS.

1.2 OWNERS TESTING AGENCY RESPONSIBILITIES

1.2.1 FOUNDATION SOIL INSPECTION TO INCLUDE VERIFICATION OF UNDERCUTTING, ALLOWABLE SOIL BEARING CAPACITY AND COMPACTION TESTING.

2.0 FOUNDATION SOILS

2.1 MATERIALS AND REQUIREMENTS

2.1.1 ALL FOUNDATION BEARING SOILS SHALL BE UNDERCUT A MINIMUM OF 4" (OR GREATER) AND REPLACED WITH FLOWABLE FILL. FINAL DEPTH OF UNDERCUT SHALL BE DETERMINED IN THE FIELD BY THE OWNERS GEOTECHNICAL ENGINEER.

3.0 RAMP WALL BACKFILL

3.1 MATERIALS AND REQUIREMENTS

3.1.1 ONSITE SOILS CLASSIFYING AS ML, SM, SC, CL, OR BETTER MAY BE USED TO BACKFILL RAMP WALLS.

3.1.2 ALL SOILS SHALL BE PLACED IN LOOSE LIFTS NOT GREATER THAN 8 INCHES AND COMPACTED TO AT LEAST 95% MAXIMUM DRY DENSITY AS DETERMINED BY ASTM D4958 STANDARD PROCTOR METHOD.

3.1.3 COMPACTION EQUIPMENT SHALL CONSIST OF LIGHT HAND OPERATED PLATE TAMPERS OR SMALL WALK-BEHIND ROLLERS.

PART 2 - CONCRETE

WORK ON ACACIA LODGE NO. 10 A.F. & A.M. SHALL CONFORM TO ALL REQUIREMENTS OF ACI 301-05 PUBLISHED BY THE AMERICAN CONCRETE INSTITUTE, FARMINGTON HILLS, MICHIGAN, EXCEPT AS MODIFIED BY THESE CONTRACT DOCUMENTS.

1.0 GENERAL REQUIREMENTS

1.0 QUALITY ASSURANCE

1.0.3 TESTING RESPONSIBILITIES OF CONTRACTOR

1.0.3.2.b CONTRACTOR IS RESPONSIBLE FOR SUPPLYING CONCRETE STRENGTH TEST CYLINDER CURING BOX

1.0.3.2.c ADVISE OWNERS TESTING AGENCY (DOMINION ENGINEERING ASSOCIATES) AT LEAST 24 HOURS IN ADVANCE OF OPERATIONS TO ALLOW FOR COMPLETION OF QUALITY TESTS AND FOR ASSIGNMENT OF PERSONNEL.

1.0.4 TESTING RESPONSIBILITIES OF TESTING AGENCY - (DOMINION ENGINEERING ASSOCIATES) (DEA)

1.0.4.2 TESTING AND INSPECTION SERVICES - SHALL INCLUDE THE FOLLOWING: REINFORCEMENT SIZE, STRENGTH, LOCATION, SUPPORT, CONDITION, ETC; CONCRETE TESTING TO INCLUDE TEMPERATURE, BATCH TO PLACEMENT TIME, SLUMP AND CONCRETE STRENGTH TEST CYLINDER SAMPLING.

1.0.4.2.4 TESTING AGENCY SHALL OBTAIN SAMPLES IN ACCORDANCE WITH ASTM C 172. OBTAIN AT LEAST ONE SET OF CYLINDERS EACH DAY OF CONCRETE PLACEMENT. ONE SET OF CYLINDERS SHALL CONSIST OF TWO 7-DAY, TWO 28-DAY, TWO 90-DAY CYLINDERS.

1.0.4.2.1 TESTING AGENCY SHALL DETERMINE SLUMP OF EACH SET OF CYLINDERS IN ACCORDANCE WITH ASTM C 1430 1430.

1.0.4.2.2 TESTING AGENCY SHALL DETERMINE TEMPERATURE OF EACH SET OF CYLINDERS IN ACCORDANCE WITH ASTM C 1064 1064.

1.0.7 ACCEPTANCE OF CONCRETE STRENGTH

1.0.7.1 STANDARD MOIST AND CURED STRENGTH SPECIMENS - THE STRENGTH LEVEL OF CONCRETE WILL BE CONSIDERED SATISFACTORY WHEN STRENGTH TEST RESULTS, PERFORMED IN ACCORDANCE WITH ASTM C 318 318, EQUAL OR EXCEED f_c FOR STRENGTH TESTS NOT MEETING f_c REQUIREMENTS PLEASE SEE SECTION 1.7 FOR BRINGING CONCRETE INTO COMPLIANCE.

1.7 ACCEPTANCE OF STRUCTURE

1.7.1 GENERAL

1.7.1.5 CONTRACTOR SHALL PAY ALL COSTS TO BRING CONCRETE WORK INTO COMPLIANCE WITH REQUIREMENTS OF PROJECT SPECIFICATION. FAILURE TO MEET SPECIFICATIONS FOR f_c , SLUMP, TEMPERATURE OR BATCH-TO-PLACEMENT TIME WILL RESULT IN FURTHER EVALUATION OF f_c IN ACCORDANCE WITH ACI SPECIFICATIONS FOR STRUCTURAL CONCRETE, SECTION 1.8.4, 1.8.2, 1.8.8. ALL FURTHER EVALUATION OTHER THAN THAT DEFINED IN SECTION 1.0.4.2 OF THESE SPECIFICATIONS WILL BE COSTS INCURRED BY THE CONTRACTOR AND NOT THE RESPONSIBILITY OF THE OWNER.

2.0 FORMWORK AND FORMWORK ACCESSORIES

2.1 GENERAL

2.1.1 DESCRIPTION - FOUNDATION FORMS SHALL BE PLACED CONSISTENT WITH THE EXTERIOR DIMENSIONS OF THE EXISTING LOOGE. DIMENSIONS SHOWN WITHIN THESE PLANS ARE FOR REFERENCE ONLY. DEA TAKES NO RESPONSIBILITY OF REQUIRED FINAL DIMENSIONS. SURVEY CONTROL BY OTHERS.

2.2 PRODUCTS

2.2.2 PERFORMANCE AND DESIGN REQUIREMENTS

2.2.2.2 EARTH CUTS MAY BE USED AS FORMS. HOWEVER, IF EARTH CUTS CANNOT BE MAINTAINED VERTICAL, PLYWOOD OR METAL FORMS ARE REQUIRED WITH PROPER SUPPORT MECHANISMS. LOOSE EARTH IS NOT PERMITTED ON BOTTOM EARTH FORM SURFACE.

3.0 REINFORCEMENT AND REINFORCEMENT SUPPORTS

3.2 PRODUCTS

3.2.1 MATERIALS

3.2.1.1 REINFORCING BARS - ALL REINFORCEMENT SHALL COMPLY WITH ASTM A 615A 615A.

3.3 EXECUTION

3.3.2 PLACEMENT

3.3.2.1 REINFORCEMENT SHALL BE PLACED AND SECURED IN ACCORDANCE WITH THOSE REQUIREMENTS IN ACI 117. "WET STICKING" OF REINFORCEMENT IS NOT ACCEPTABLE IN ANY CIRCUMSTANCES.

3.3.2.4 REINFORCEMENT SUPPORTS - ONLY WIRE CHAIRS WITH PLASTIC TIPS, ALL PLASTIC CHAIRS OR PRECAST CONCRETE SUPPORTS ARE PERMITTED.

3.3.2.7 SPLICES - NO. 4 REINFORCING BARS SHALL BE SPLICED A MINIMUM OF 24 INCHES. NOT MORE THAN 50% OF THE REINFORCEMENT SHALL BE SPLICED AT THE SAME LOCATION WITHIN A STRUCTURAL ELEMENT.

4.0 CONCRETE MIXTURES

4.1 GENERAL

4.1.2 SUBMITTALS

4.1.2.1 MIXTURE PROPORTIONS - SUBMIT CONCRETE MIXTURE PROPORTIONS AND CHARACTERISTICS TO DOMINION ENGINEERING ASSOCIATES FOR REVIEW AND APPROVAL AT LEAST TWO BUSINESS WEEKS PRIOR TO CONSTRUCTION.

4.2 PRODUCTS

4.2.2 PERFORMANCE AND DESIGN REQUIREMENTS

4.2.2.2 SLUMP - SHALL BE BETWEEN 3 INCHES AND 5 INCHES AT THE TIME OF PLACEMENT.

4.2.2.3 SIZE OF COARSE AGGREGATE - SHALL NOT EXCEED $\frac{1}{4}$ ".

4.2.2.4 AIR CONTENT

4.2.2.4.a FOOTING CONCRETE MIXTURE SHALL HAVE A MINIMUM AIR CONTENT OF 3.2% AND A MAXIMUM AIR CONTENT OF 5%.

4.2.2.4.b FOUNDATION WALL, CONCRETE AND EXTERIOR SLABS SHALL HAVE A MINIMUM AIR CONTENT OF 5% AND A MAXIMUM AIR CONTENT OF 7.5%.

4.2.2.5 ADMIXTURES ARE NOT ACCEPTABLE UNLESS SUBMITTED TO AND APPROVED BY DOMINION ENGINEERING ASSOCIATES (DEA).

4.2.2.6 CHLORIDE-ION CONCENTRATION - MAXIMUM WATER SOLUBLE CHLORIDE-ION CONCENTRATION IN HARDENED CONCRETE AT AGES FROM 28 TO 42 DAYS, CONTRIBUTED BY ALL MATERIALS USED IN CONCRETE MIX, SHALL NOT EXCEED 0.15% BY WEIGHT OF CEMENT.

4.2.2.8 STRENGTH AND WATER-CEMENTITIOUS MATERIAL RATIO - THE 28 DAY COMPRESSIVE STRENGTH (f_c) SHALL BE NO LESS THAN 3,000 PSI AND HAVE REACHED AT LEAST 70% OF f_c AT 7 DAYS. THE CONCRETE MIX SHALL NOT HAVE A WATER-CEMENTITIOUS RATIO EXCEEDING 40.

4.2.2.8.a EXTERIOR SLAB CONCRETE MIXTURES SHALL FOLLOW THOSE MAXIMUM CEMENTITIOUS MATERIAL REQUIREMENTS SPECIFIED IN ACI 301-05, TABLE 4.2.2.2.

4.2.2.8.b CONCRETE FOUNDATIONS SHALL NOT BE LOADED (REPLACING LOOGE) UNTIL 80% OF f_c IS VERIFIED BY CONCRETE COMPRESSIVE STRENGTH TESTING. SEE SECTION 1.0.7 OF THESE SPECIFICATIONS FOR FURTHER DETAILS.

5.0 HANDLING, PLACING AND CONSTRUCTING

5.3 EXECUTION

5.3.1 PREPARATION

5.3.1.4 BEFORE PLACING CONCRETE:

- SUBGRADE SHALL BE WELL DRAINED AND MEET THOSE REQUIREMENTS SPECIFIED IN PART 1 - SOILS OF THESE SPECIFICATIONS.
- SUBGRADE SHALL BE FREE OF FROST, ICE OR WATER.
- ALL REINFORCEMENT SHALL BE INSTALLED AND SECURED TO PREVENT DISPLACEMENT DURING CONCRETE PLACEMENT.

5.3.2 PLACEMENT OF CONCRETE

5.3.2.5 CONSOLIDATING - CONSOLIDATION OF CONCRETE IS MANDATORY. CONSOLIDATE CONCRETE BY VIBRATION AROUND REINFORCEMENT AND INTO CORNERS OF FORMS. INTERNAL VIBRATORS SHALL BE USED FOR FOUNDATION WALLS. DO NOT USE VIBRATORS TO MOVE CONCRETE WITHIN THE FORMS.

5.3.6 CURING AND PROTECTION

5.3.6.5 PROTECTION - IMMEDIATELY AFTER PLACEMENT, PROTECT CONCRETE FROM PREMATURE DRYING, EXCESSIVELY HOT AND COLD TEMPERATURES AND MECHANICAL INJURY. ACI HOT AND COLD WEATHER CONCRETING TECHNIQUES SHALL BE FOLLOWED.

PART 3 - STRUCTURAL STEEL

1.0 STEEL SHAPES

1.1 W SHAPES: ASTM A992 (GRADE 50)

1.2 ANGLES: ASTM A36

2.0 ANCHORS AND BOLTS

2.1 ANCHOR BOLTS: ASTM F1554, GRADE 36, ANCHOR BOLTS WITH A 90° BEND.

2.1.1 ANCHOR BOLTS SHALL BE EMBEDDED INTO FOUNDATION WALL A MINIMUM OF 7 INCHES. THERE SHALL BE AT LEAST THREE ANCHOR BOLTS PER PLATE SECTION WITH ONE BOLT LOCATED BETWEEN 3 TO 8 INCHES FROM THE END OF THE PLATE SECTION.

2.2 BOLTS: #3 ASS BOLTS

2.2.1 BOLTS SHALL BE TORQUED TO A MINIMUM OF 150 Ft-lb. BOLTED CONNECTIONS ARE DESIGNED AS NON-SLIP CRITICAL JOINTS, BUT SHALL HAVE PROPER INSTALLATION TORQUE APPLIED FOR EASE OF SPECIAL INSPECTION.

3.0 STRUCTURAL STEEL SHALL BE FABRICATED AND ERECTED ACCORDING TO THE "ALLOWABLE STRESS DESIGN SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS" AND THE AISI "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES", EXCEPT WHERE MODIFIED BY THESE STRUCTURAL DESIGN DRAWINGS AND SPECIFICATIONS.

4.0 CONTRACTOR IS RESPONSIBLE FOR LAYOUT AND FINAL DIMENSIONS OF STRUCTURAL STEEL. DIMENSIONS SHOWN WITHIN THESE PLANS ARE FOR REFERENCE ONLY. FINAL DIMENSIONS WILL BE BASED ON THOSE MEASURED "EXACTLY" IN THE FIELD.

5.0 SHOP DRAWINGS SHALL BE SUBMITTED TO DOMINION ENGINEERING ASSOCIATES FOR REVIEW AND APPROVAL. SUBMITTAL SHALL ADEQUATELY DEPICT FINAL DIMENSIONS, STRUCTURAL MEMBERS AND CONNECTIONS.

6.0 WELD ELECTRODES SHALL BE ERXX ELECTRODES. ALL WELDS SHALL BE PERFORMED IN ACCORDANCE WITH AISC AND THE STRUCTURAL WELDING CODE OF THE AMERICAN WELDING SOCIETY (AWS D1.1).

7.0 FABRICATOR SHALL AT A MINIMUM, HAND CLEAN THE STRUCTURAL STEEL PRIOR TO SHOP PAINTING. SHOP PAINTING SHALL PROVIDE A MINIMUM OF ONE MIL DRY FILM THICKNESS.

8.0 QUALITY ASSURANCE AND INSPECTIONS

8.1 TESTING AND INSPECTION RESPONSIBILITIES OF CONTRACTOR

8.1.1 ADVISE TESTING AND INSPECTION AGENCY (DOMINION ENGINEERING ASSOCIATES) AT LEAST 24 HOURS IN ADVANCE OF OPERATIONS TO ALLOW FOR COMPLETION OF QUALITY TESTS AND FOR ASSIGNMENT OF PERSONNEL.

8.2 TESTING AND INSPECTION RESPONSIBILITIES OF TESTING/INSPECTION AGENCY

8.2.1 TESTING AND INSPECTIONS SHALL BE PERFORMED TO ENSURE CONFORMANCE WITH CONTRACT DOCUMENTS. INSPECTIONS AND TESTING SHALL INCLUDE BUT NOT LIMITED TO:

- VISUAL WELD INSPECTIONS IN ACCORDANCE TO AWS D1.1
- BOLT TORQUE
- MATERIAL SIZE, STRENGTH (GRADE) AND LOCATION VERIFICATION
- STEEL FIT UP

PART 4 - WOOD

1.0 GENERAL

1.1 EVALUATION OF EXISTING WOOD FRAMING - DOMINION ENGINEERING ASSOCIATES AND THE CONTRACTOR SHALL EVALUATE THE EXISTING FRAMING IN THOSE AREAS PROPOSED FOR MODIFICATION WHERE FRAMING IS EXPOSED AND CAN BE OBSERVED. FURTHER RECOMMENDATIONS WILL MOST LIKELY BE REQUIRED ONCE TYPE AND INTEGRITY OF WOOD FRAMING IS UNCOVERED AND EVALUATED.

1.2 WOOD CONSTRUCTION SHALL CONFORM TO CHAPTER 23, OF THE INTERNATIONAL BUILDING CODE (UNLESS OTHERWISE NOTED). ALL FASTENING SHALL CONFORM TO TABLE 2304.9.1 "FASTENING SCHEDULE" OF THE INTERNATIONAL BUILDING CODE UNLESS OTHER REQUIREMENTS ARE NOTED ON THE PLANS OR WITHIN THESE SPECIFICATIONS.

2.0 MATERIALS

2.1 WOOD FRAMING

2.1.1 LVL HEADERS -

MINIMUM DESIGN VALUES:

$F_b = 2,700$ psi - BENDING

$F_v = 200$ psi - SHEAR

$E = 1,800,000$ psi - MODULUS OF ELASTICITY

2.1.2 HEADERS - ALL EXTERIOR WALL HEADERS NOT SPECIFIED WITHIN PLANS SHALL CONSIST OF A MINIMUM OF 2 - 2"x10" NO. 2 SOUTHERN YELLOW PINE (SYP)

MINIMUM DESIGN VALUES:

$F_b = 1,600$ psi - BENDING

$F_v = 175$ psi - SHEAR

$E = 1,800,000$ psi - MODULUS OF ELASTICITY

2.1.3 FLOOR JOISTS - NO. 2 SYP 2"x8"

MINIMUM DESIGN VALUES:

$F_b = 1,200$ psi - BENDING

$F_v = 175$ psi - SHEAR

$E = 1,800,000$ psi - MODULUS OF ELASTICITY

2.1.4 STUDS, PLATES, BRACING, STIFFENERS, JACKS, BLOCKING, ETC. - MINIMUM NO. 2 GRADE

MINIMUM DESIGN VALUES:

$F_b = 825$ psi - BENDING

$F_v = 65$ psi - SHEAR

$F_c (parallel) = 1,200$ psi - COMPRESSION PARALLEL TO THE GRAIN

$F_c (perp) = 625$ psi - COMPRESSION PERPENDICULAR TO THE GRAIN

$E = 1,800,000$ psi - MODULUS OF ELASTICITY

2.1.5 ROOF AND EXTERIOR WALL (BRACED WALL LINE) SHEATHING - $\frac{3}{4}$ " APA EXPOSURE 1, RATED SHEATHING WITH 32/16 SPAN RATING

2.1.6 FLOOR SHEATHING - $\frac{3}{4}$ " APA EXPOSURE 1, RATED SHEATHING WITH A 4024 SPAN RATING

2.1.7 PRESERVATIVE TREATED LUMBER - ALL PRESERVATIVE TREATED LUMBER SHALL MEET AWPA C2 OR C8 STANDARDS.

2.2 MISCELLANEOUS CONNECTORS AND FASTENERS

2.2.1 BRACKING AND GABLE WALL TENSION TIES (SEE DETAIL 4 ON PAGE 3) SHALL CONSIST OF SIMPSON® 1000 TENSION BRIDGING OR EQUIVALENT.

2.2.2 TRUSS UPLIFT CONNECTIONS SHALL INCLUDE ONE SIMPSON® HS UPLIFT CONNECTOR (OR EQUIVALENT) PER TRUSS REACTION AT A MINIMUM.

3.0 NON-PRESPECIFIED FASTENING REQUIREMENTS

3.1 LVL BEAMS AND HEADERS WITH MULTIPLE MEMBERS SHALL BE FASTENED WITH 3 - 1/2" NAILS AT 6" ON CENTER EACH SIDE.

3.2 DIMENSIONAL LUMBER BEAMS AND HEADERS WITH MULTIPLE MEMBERS SHALL BE FASTENED WITH 3 - 1/2" NAILS AT 6" ON CENTER EACH SIDE.

3.3 EXTERIOR WALL SHEATHING (BRACED WALLS) SHALL BE FASTENED WITH 8d NAILS AT 6" ON CENTER AT ALL SUPPORTED EDGES AND 12" ON CENTER ALONG INTERMEDIATE FRAMING MEMBERS (FIELD OF PANEL).

4.0 WOOD TRUSS REQUIREMENTS

4.1 MANUFACTURED WOOD TRUSSES SHALL CONFORM WITH THOSE REQUIREMENTS SPECIFIED WITHIN THE INTERNATIONAL BUILDING CODE SECTION 2303.4.

4.2 TRUSS DESIGN DRAWINGS SHALL MEET THE REQUIREMENTS OF THE INTERNATIONAL BUILDING CODE SECTION 2303.4.1 AND BE SUBMITTED TO DOMINION ENGINEERING ASSOCIATES FOR REVIEW AND APPROVAL.

5.0 QUALITY ASSURANCE AND INSPECTIONS

5.1 INSPECTION RESPONSIBILITIES OF CONTRACTOR

5.1.1 ADVISE INSPECTION AGENCY (DOMINION ENGINEERING ASSOCIATES) AT LEAST 24 HOURS IN ADVANCE OF OPERATIONS TO ALLOW FOR COMPLETION OF INSPECTIONS AND FOR ASSIGNMENT OF PERSONNEL.

5.2 INSPECTION RESPONSIBILITIES OF INSPECTION AGENCY

5.2.1 INSPECTIONS SHALL BE PERFORMED TO ENSURE CONFORMANCE WITH CONTRACT DOCUMENTS. INSPECTIONS SHALL INCLUDE BUT NOT LIMITED TO:

- WOOD FRAMING COMPONENT SPECIES, GRADE AND CONDITION VERIFICATION
- FASTENING SCHEDULE VERIFICATION
- WOOD FRAMING COMPONENT SIZES, SPACING AND AMOUNT VERIFICATION
- WOOD TRUSS LAYOUT, CONNECTION AND BRACING VERIFICATION



DOMINION ENGINEERING ASSOCIATES, INC.
5110 SOUTHPOINT PARKWAY
FREDERICKSBURG, VA 22407

ACACIA LODGE NO. 10 A.F. & A.M.
CLYTON, VIRGINIA
SPECIFICATIONS

DATE	4 AUG 2008	SHEET	5 OF 5	SCALE	NONE
DESIGNED BY	JD	DRAWN BY	JB	CHECKED BY	RH